

Reinforced Concrete Beam

Simple-span rectangular beam under uniform gravity load. Numbers come from calc.py. This sheet only presents them.

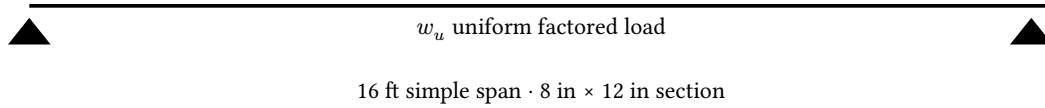


Figure 1: 16 ft simply supported beam, 8 in × 12 in rectangular section.

Section and Reinforcement

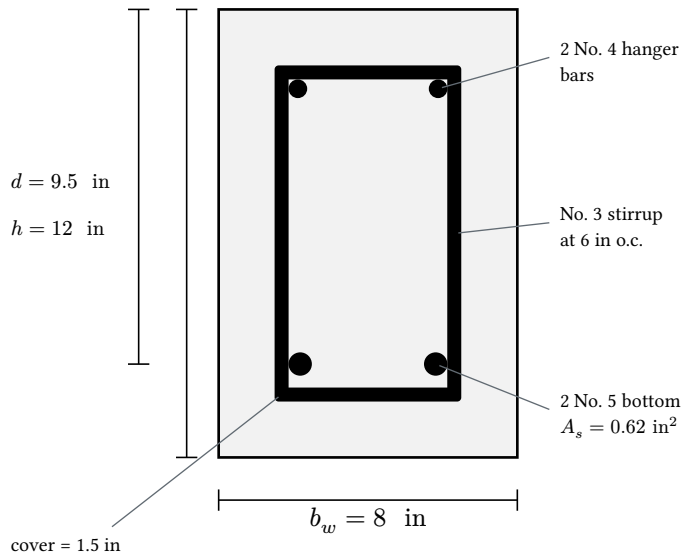


Figure 2: Beam cross-section: 8 in × 12 in, No. 3 stirrups at 6 in on center, and 2 No. 5 longitudinal bars ($A_s = 0.62$ in²).

Loads and Beam Demand

$L = 16$ ft	Simple span
$B_t = 6.25$ ft	Tributary width
$D = 55$ psf	Dead load including superimposed dead
$L_L = 20$ psf	Live load
$w_{sw} = 0.1$ kip/ft	Beam self-weight
$w_u = 1.2w_D + 1.6w_L = 0.733$ kip/ft	Factored uniform line load
$M_u = w_u \frac{L^2}{8} = 23.44$ kip·ft	Maximum positive moment
$V_u = w_u \frac{L}{2} = 5.86$ kip	Support shear

Flexural Strength

$$b_w = 8 \text{ in}$$

Beam width

$$h = 12 \text{ in}$$

Overall depth

$$d = 9.5 \text{ in}$$

Effective depth

$$f'_c = 3 \text{ ksi}$$

Concrete compressive strength

$$f_y = 60 \text{ ksi}$$

Steel yield strength

$$A_s = 0.62 \text{ in}^2$$

Provided tension steel (2 No. 5)

$$a = A_s \frac{f_y}{0.85 f'_c b_w} = 1.824 \text{ in}$$

Equivalent compression-block depth

$$\varepsilon_t = 0.0103$$

Net tensile strain

$$\varphi = 0.9$$

Strength reduction factor

$$\varphi M_n = \varphi A_s f_y \left(d - \frac{a}{2} \right) = 23.96 \text{ kip}\cdot\text{ft}$$

Design flexural strength

Flexural strength

OK

$$M_u: 23.44 \text{ kip}\cdot\text{ft} \quad \varphi M_n: 23.96 \text{ kip}\cdot\text{ft}$$

D/C: 0.98

Minimum Steel and Concrete Shear

$$A_{s,\min} = 0.253 \text{ in}^2$$

Minimum longitudinal steel

$$A_{s,\text{prov}} = 0.62 \text{ in}^2$$

Provided longitudinal steel

Minimum longitudinal reinforcement

OK

$$A_{s,\min}: 0.25 \text{ in}^2 \quad A_{s,\text{prov}}: 0.62 \text{ in}^2$$

D/C: 0.41

$$V_c = 2\sqrt{f'_c} b_w d = 8.33 \text{ kip}$$

Concrete shear strength

$$\varphi V_c = 6.24 \text{ kip}$$

Design concrete shear strength

Concrete shear

OK

$$V_u: 5.86 \text{ kip} \quad \varphi V_c: 6.24 \text{ kip}$$

D/C: 0.94